

## Memorandum

date March 19, 2007

to Kamuron Gurol, City of Sammamish

prepared by Alex Cohen, ESA Adolfson, Mike Birdsall, David Evans Associates, and Dan McKinney, Jr., The Transpo Group

### **Sammamish Town Center Sub-Area Plan**

#### **Technical Memorandum on the DEIS Transportation Analysis**

The Draft Environmental Impact Statement (DEIS) for the proposed Sammamish Town Center Sub-Area Plan was issued on January 31, 2007. Since the date of issuance several Planning Commissioners, City Council members, and citizens have raised questions regarding the methods and data presented in the transportation analysis (Chapter 7). City staff held two public meetings on February 21, 2007 and March 9, 2007 at which the public (including planning commissioners and council members) were invited to ask questions of the City transportation staff, the city's transportation consultants, (David Evans and Associates (DEA)), and the DEIS consultant team (ESA Adolfson, EIS lead and The Transpo Group (Transpo), DEIS Transportation analysis lead).

At these meetings several clarifying questions were raised regarding the methods behind the transportation analysis and the conclusions. Several written questions were also submitted to the City. To the extent possible, city staff and the consultant team addressed these questions at the meetings. Where further inquiry or adjustment were required city staff and the consultant team agreed to research questions, refine data, and /or add further explanations for elements of concern.

The purpose of this memo is to present these questions and to provide supplemental data, adjustments to the analysis or expanded explanations for the issues raised. The organization of this memo follows the topics that were raised. Each issue is presented as a bold header and followed by a response in the text. There are also several attachments, which are referred to under particular topics..

#### **1. Variations in Daily Tube Counts**

Concerns have been raised about differences in traffic counts from year to year, and even within the same year from different sources. Consistent with standard industry best practices, traffic engineers understand that traffic counts at the same location will vary by as much as 10% from day to day for the reason that people's daily activities are not the same every day. Traffic varies due to factors such as special events, weather conditions, and traffic conditions elsewhere. Within a year, seasonal variations may be 10% to 25% in urban areas, and higher in



rural areas. To reduce the uncertainty associated with counts, it is typical to take the average of 2 to 3 day's counts, or count for an entire week and compute the weekly average. But that is not always done, for economy. Counts at different points along a road (e.g., East Lake Sammamish Parkway between Sammamish and Redmond) also may vary due to turning activity along the road between those points, so care must be taken to adjust for intermediate events when comparing counts at different locations.

The trend of historical counts at the same location does not always change at the same rate as the surrounding area as a whole. A highly congested road may show little change from year to year in spite of area-wide growth because there is little capacity to accept more growth. When that happens, it is likely that a parallel road will exhibit above-average growth. The sum of both roads' growth would tend toward the average for the area.

## **2. Section 36 Park Trip Generation**

As explained at the March 9, 2007 meeting, the disparity of volumes on Trossachs Boulevard is not related to development activity but to an inconsistency in the handling of future user activity at the Section 36 park. This is easily corrected in the traffic model, to obtain consistent volumes on Trossachs Boulevard for all cases. It will also add some traffic volumes to various roads citywide, dissipating with distance from Trossachs Boulevard.

## **3. Updated Traffic Volume Figures:**

It was noted that the volume ratios between the PM peak hour and the daily volumes fluctuated from alternative to alternative. The small variations were due to rounding; and the larger variations were due to reporting the PM volumes from a slightly different segment of the link than from the location that daily volumes were reported. These figures were updated to report the PM peak hour volume for the same location that the daily volumes were generated from. The updated figures are provided in **Attachment A**.

## **4. Relative Impacts of the Alternatives**

### **Total Trip Distribution**

Attached (**Attachment B**) are three small figures showing the flow of trips generated by each DEIS alternative for the Town Center site. These show the distribution of travel to and from the Town Center site. The scale of each of the three figures is the same, so relative comparisons between the figures are reasonably good indicators of different volume magnitudes.

Please note: The direct impact of Town Center in the figures in **Attachment B** is a larger number at some locations than the net change from No Action for the same alternative. This is a manifestation of Town Center trips being internalized within Sammamish. As some Town Center trips are assigned to destinations within Sammamish, they displace other Sammamish-based trips. Traffic distributions for all zones in Sammamish are affected by Town Center. Some trips at other zones are redistributed, citywide, due to the new opportunities provided by the land use in Town Center.

Therefore, simply adding the direct impact of any alternative to the No Action base forecast tends to over-predict total future demand, especially at the fringes of the city. That is not usually an issue for individual developments. But with a planned area of this magnitude, the redistribution effects within Sammamish are significant. The traffic model addresses that automatically. The reader is advised to use the direct impact plots for a general impression of where Town Center trips go. Use the net difference between cases for the net impact.



### Trip Generation Analysis

The following tables provide more detailed trip generation summaries for each Town Center alternative than was presented in the DEIS. Specifically, this provides the breakdown of trips generated by each general land use category and provides the inbound and outbound split. Trip generation rates within each general land use category include a variety of subtypes, which differ somewhat between the alternatives. For example; the residential category includes single-family and multi-family dwellings; the retail category includes everything from gas stations and fast food restaurants to specialty stores, drug stores, and supermarkets; the office land use accounts for all types of non-retail employment; and open space is a general category used in the traffic model to represent parks, playgrounds, etc. The open space trip allowance is a constant in the traffic model for all three alternatives.

The total trips reported for each land use alternative is larger than previously reported in the DEIS because in this format each trip that remains within Town Center is counted twice – once outbound and once inbound. The summary in the DEIS didn't accurately account for this. There has been no change in the actual amount of trip generation to the external roadway network; only the manner of reporting has changed. The main impact of this change is that the percentage of trips internalized is larger than previously described, and more consistent with the level commonly expected for multi-use developments. Beyond the boundaries of Town Center, all trips are the same as previously reported.

Trip generation summaries are presented in various ways, to answer particular interests at the boundary of Town Center, versus the boundaries of the City of Sammamish. Directional splits in and out of the developments are provided, which show the difference in directionality of residential trip generation versus office generation or retail generation, in the afternoon peak hour.

### Alternative 1 Trip Generation Summary

#### PM Peak Hour Trip Generation:

| Land Use           | Amount | Units      | Out-bound | In-bound | Total | Share | Out-bound | In-bound |
|--------------------|--------|------------|-----------|----------|-------|-------|-----------|----------|
| Residential        | 3,514  | dwellings  | 717       | 1247     | 1964  | 28%   | 37%       | 63%      |
| Retail             | 530    | 1,000 s.f. | 2074      | 1894     | 3968  | 56%   | 52%       | 48%      |
| Office             | 416    | 1,000 s.f. | 378       | 125      | 503   | 7%    | 75%       | 25%      |
| Open Space         | 550    | Trips      | 325       | 301      | 627   | 9%    | 52%       | 48%      |
| <b>Total Trips</b> |        |            | 3495      | 3567     | 7062  | 100%  | 49%       | 51%      |

#### Trip Distribution by Major Areas (from trip table):

|                         |      |      |      |      |     |     |
|-------------------------|------|------|------|------|-----|-----|
| Within Town Center      | 1374 | 1374 | 2748 | 39%  | 50% | 50% |
| To/From Sammamish Other | 1391 | 1209 | 2600 | 37%  | 54% | 47% |
| To/From External Areas  | 730  | 984  | 1714 | 24%  | 43% | 57% |
| <b>Total Trips</b>      | 3495 | 3567 | 7062 | 100% | 49% | 51% |

Total Trips without double-count of "within" trips: 5688

|  |      |     |     |
|--|------|-----|-----|
| Net trip generation leaving Town Center: | 4314 | 49% | 51% |
| Net trip generation leaving Sammamish:   | 1714 | 43% | 57% |



## Alternative 2 Trip Generation Summary

### Afternoon Peak Hour Trip Generation:

| Land Use    | Amount | Units      | Out-bound | In-bound | Total | Share | Out-bound | In-bound |
|-------------|--------|------------|-----------|----------|-------|-------|-----------|----------|
| Residential | 1,104  | dwellings  | 308       | 474      | 782   | 26%   | 39%       | 61%      |
| Retail      | 167    | 1,000 s.f. | 782       | 691      | 1473  | 50%   | 53%       | 47%      |
| Office      | 30     | 1,000 s.f. | 34        | 8        | 42    | 1%    | 80%       | 20%      |
| Active Land | 550    | Trips      | 349       | 315      | 663   | 22%   | 53%       | 47%      |
| Total Trips |        |            | 1472      | 1488     | 2960  | 100%  | 50%       | 50%      |

### Trip Distribution by Major Areas (from trip table):

|                         |      |      |      |      |     |     |
|-------------------------|------|------|------|------|-----|-----|
| Within Town Center      | 376  | 376  | 752  | 25%  | 50% | 50% |
| To/From Sammamish Other | 845  | 713  | 1558 | 53%  | 54% | 46% |
| To/From External Areas  | 251  | 399  | 650  | 22%  | 39% | 61% |
| Total Trips             | 1472 | 1488 | 2960 | 100% | 50% | 50% |

Total Trips without double-count of "within" trips: 2584

|  |      |
|--|------|
| Net trip generation leaving Town Center: | 2208 |
| Net trip generation leaving Sammamish:   | 650  |

|     |     |
|-----|-----|
| 50% | 50% |
| 39% | 61% |

## Alternative 3 Trip Generation Summary

### Afternoon Peak Hour Trip Generation:

| Land Use    | Amount | Units      | Out-bound | In-bound | Total | Share | Out-bound | In-bound |
|-------------|--------|------------|-----------|----------|-------|-------|-----------|----------|
| Residential | 2,961  | dwellings  | 635       | 1084     | 1719  | 36%   | 37%       | 63%      |
| Retail      | 254    | 1,000 s.f. | 1147      | 1026     | 2173  | 45%   | 53%       | 47%      |
| Office      | 200    | 1,000 s.f. | 183       | 59       | 242   | 5%    | 75%       | 25%      |
| Active Land | 550    | Trips      | 344       | 313      | 657   | 14%   | 52%       | 48%      |
| Total Trips |        |            | 2309      | 2482     | 4791  | 100%  | 48%       | 52%      |

### Trip Distribution by Major Areas (from trip table):

|                         |      |      |      |      |     |     |
|-------------------------|------|------|------|------|-----|-----|
| Within Town Center      | 871  | 871  | 1742 | 36%  | 50% | 50% |
| To/From Sammamish Other | 1060 | 919  | 1979 | 41%  | 54% | 46% |
| To/From External Areas  | 378  | 692  | 1070 | 22%  | 35% | 65% |
| Total Trips             | 2309 | 2482 | 4791 | 100% | 48% | 52% |

Total Trips without double-count of "within" trips: 3920

|  |      |
|--|------|
| Net trip generation leaving Town Center: | 3049 |
| Net trip generation leaving Sammamish:   | 1070 |

|     |     |
|-----|-----|
| 47% | 53% |
| 35% | 65% |



### **Directional Distribution of Trips within Sammamish**

The figures presented in **Attachment C** show the directional distribution pattern of trips generated in Town Center, for residential and non-residential land uses. The focus of these figures is on the trips leaving Town Center. Alternative 1 was used for these illustrations; however, the general pattern of distribution would be the same for the same land use type, in other alternatives. Total numbers of trips obviously change, but the directional patterns would be the same in a proportional sense.

The thickness of the flow patterns is proportional to volume, and the direction of travel is indicated by which side of the centerline the flow pattern is drawn. In the residential figure, the majority of travel is shown in the direction toward Town Center, since PM peak hour conditions are depicted. In the PM peak hour, roughly two-thirds of residential trip generation is inbound, toward the residence.

The non-residential distribution pattern represents a combination of retail and office developments – the mixed use concept for Town Center. The directional orientation is approximately equal in each direction, overall, but with slightly more outbound than inbound travel. Both figures are drawn to approximately the same scale, so the comparison between both figures can be used to approximately estimate the relative shares of impact on any road between the residential and non-residential developments in Town Center.

From the underlying numerical data, the relative directional distribution patterns were also summarized at three locations ranging from the edges of Town Center itself, to a mid-plateau location, and to the edges of the City of Sammamish. The share of trips oriented to the north versus the south changes depending on where the measure is taken, and whether the measure is for trips only on 228<sup>th</sup> Avenue NE/SE or on all north-south routes that carry shares of total travel.

For residential trip generation, the northward orientation of trips is 53% nearest to Town Center on 228<sup>th</sup> only, and 55% about a half-mile further away in each direction, but now counting the sum of three parallel routes. At the north and south city limits, this orientation drops to 49%. This shift is consistent with the retention of a substantial part of Town Center travel within the City of Sammamish. The higher emphasis toward the south at the city limits is consistent with the fact that commuter trips from employment elsewhere are somewhat more likely to travel via I-90 through Issaquah than via SR202 through Redmond.

The non-residential trip orientation is more pronounced toward the north, at 59% within Sammamish, and still 55% at the city limits. This is consistent with the fact that the external residential areas that will be providing employees and shoppers to future commercial developments in the Town Center are larger to the north than to the south (e.g., greater Redmond and areas from Bear Creek to Carnation, as compared to Issaquah).

The north-oriented pattern for future Town Center non-residential trips stands out as being different from the existing patterns of general traffic in Sammamish, which tends slightly more to the south than the north. This is because most Sammamish traffic today is residentially based. The residential distribution for Town Center is closer to the existing residential average for Sammamish, while the non-residential part is more north-oriented.



## 5. Response to Mr. Savage Letter

The transportation team was presented with a letter from Joe Savage, P.E. by Commissioner Hamilton and asked for a response. Many of Mr. Savage's points were addressed during the March 9, 2007 meeting and his specific points are covered in paragraph order. The letter is included as attachment F

- (a) Peak Hour to Daily Ratio of 10%. Joe has essentially agreed with Transpo and DEA that the 10% factor is not a "standard" but only a "rule of thumb" to fall back on if there is no other information to go on. The specific traffic count data available for Sammamish in years 2002 to 2006 shows a range of factors that are generally in the 8% to 9% range, and almost never match 10%. This is due to widespread congestion and associated peak-spreading.
  - (b) Estimated vs. Actual Existing Traffic. The discussion at the March 9, 2007 meeting clarified that all the "existing" data in the DEIS represents actual counts taken in 2006. Existing 2006 roadway link traffic volumes are summarized in **Attachment D**, showing the AM and PM peak hour volumes compared to the Daily volumes.
- Mr. Savage's letter recommends that "all analysis of levels of service at intersections and on street segments should be performed with peak hour rather than daily volumes." The intersection analysis was indeed done on peak hour volumes; however, consistent with City's concurrency methodology, the segment analysis was done on the daily equivalent volumes. Both methods are required to be done that way by the Comprehensive Plan as adopted City policy. There is no reason in Sammamish to do segment analysis based on peak hours.
- (c) Disagreement over Peak Hour Methods. In looking at the modeled numbers, the team can assure that the peak hour turn movements at the intersections analyzed and the peak hour link volumes posted in figures are consistent and correct. Link volumes match exactly the sum of turn movements at intersections.
  - (d) Accuracy and Validity of the Model Results. The traffic model does not use counts at all, so questions pertaining to recent counts have no bearing on the traffic model. Traffic model forecasts are derived from land use forecasts, totally independent of count data. The model was accurately calibrated to "forecast" 2001 counts based on input of 2001 land use. In the DEIS, the only use of 2006 count data is to describe existing conditions for general information. If the count data changes, that has zero effect on the traffic model forecasts. Any concerns about real world count data in 2003 to 2006 do not in any way extend to concern about the traffic model.
  - (e) Future Growth Rate may not be Sufficient. The forecast of 1% annual average growth on East Lake Sammamish Parkway is not unreasonable for that location, in context. Much more growth is forecast on 244<sup>th</sup> Avenue NE due to the future extension of that road. Figures 7-3 through 7-6 show forecast volumes that equate to 3% per year for the No Action case and as high as 5% per year with Alternative 1. The combination of both roads is consistent with the overall growth forecast for Sammamish as a whole.

For a comprehensive perspective of citywide growth rates, the following table presents data available in the City's Concurrency Monitoring System and the Town Center model forecasts, for total peak hour trip generation in Sammamish. All figures are based on the traffic model:



### TOTAL PEAK HOUR TRIP GENERATION

| Year               | Trip Generation | Annualized Growth<br>from 2006 |
|--------------------|-----------------|--------------------------------|
| 2001               | 16,510          |                                |
| 2006 (estimated)   | 18,500          |                                |
| 2013 Pipeline      | 20,845          | 1.8%                           |
| 2030 No Action     | 22,720          | 0.9%                           |
| 2030 Alternative 1 | 29,583          | 2.5%                           |
| 2030 Alternative 2 | 25,398          | 1.6%                           |
| 2030 Alternative 3 | 27,476          | 2.0%                           |

In this table, the 2030 No Action growth rate is lower than other rates, because that amount of growth is based on the current land use density assumptions in the Comprehensive Plan. Town Center alternatives would modify those policies and allow for more growth, resulting in higher average growth rates.

The current pipeline of developments in process represents the first phase of the No Action growth envelope. It appears to be “front-loaded” compared to the long-range rate to 2030 No Action. Note, however, that the year associated with pipeline developments is an artificial assumption. It is assumed to be six years ahead for planning purposes (such as calculating average growth rates for the next six years) but that is merely an assumption. The year that the pipeline growth will be 100% complete is actually at the whim of the marketplace.

(f) Model’s Reasonableness Questioned. See response in (d) above.

(g) Model Calibration. The City has a complete model calibration report, prepared by DEA’s Mike Birdsall while employed at Earth Tech. It shows that the model exceeds the expectations of the FHWA “standards” by a large margin. This calibration information was presented to the Ad-Hoc Planning Advisory Committee in 2002 and was part of the process of establishing credibility of the model and model forecasts that supported the adoption of the Comprehensive Plan Transportation Element in 2002.

(h) Quick Check on Model Validity. The model calibration report includes just such a table of screenlines, showing the model to be within 2% to 5% of actual counts in 2002. We agree that the screenline technique is a useful way to summarize traffic trends. If necessary, it could be incorporated into the FEIS, as an additional way to view and understand in proper context the data already provided.

## 6. Comparison of AM and PM Peak Hour Traffic

The City of Sammamish collected updated traffic volumes throughout the City in late February and early March 2006. Specifically the data was collected February 28<sup>th</sup> through March 2<sup>nd</sup>. A figure summarizing the average AM peak hour, PM peak hour, and Daily counts is provided in **Attachment A**. As shown in the figure, all of the PM peak hour volumes exceed the AM peak hour volumes with the exception of one location. The AM peak hour traffic volumes on 244<sup>th</sup> Avenue NE, just south of SR 202 (NE Redmond Fall City Road), are slightly higher. The remainder of the City has higher traffic volumes occurring during the PM peak hour. Since traffic volumes are typically highest during the PM peak hour, the City’s traffic model and concurrency program have been developed around the PM peak hour.



The focus of the analysis was based on the PM peak hour, as the combination of traffic generated by any of the Town Center land use alternatives along with the adjacent street traffic would be at the highest levels during the PM peak hour.

Intersection levels of service were evaluated for both the AM and PM peak hours in the Eastside Catholic EIS analysis. Although the AM peak hour volumes are lower than the PM peak hour volumes, there are some locations where the level of service is worse during the AM peak hour. This occurs most notably along Eastlake Sammamish Parkway at SE 56<sup>th</sup> Street, Inglewood Hill Road, and SR 202. This is due to the large volume of traffic heading off the Sammamish Plateau funneling toward Redmond. The PM volumes are still higher than the AM peak hour due to a more balanced flow of volumes in both directions. The existing level of service results and volumes reported in the Eastside Catholic High School EIS are provided in **Attachment E**.

## 7. Traffic Counts and Future Forecast Modeling

Commissioner Hamilton asked for clarification of the message that “numbers don’t matter, only land use matters” for the modeling, which appears to contrast with his understanding that traffic counts are a key component of concurrency and traffic mitigation impact fees. Part of the answer is to differentiate carefully between different kinds of traffic numbers. All numbers are not created equal. Traffic “numbers” in a report may be of several kinds:

- Actual counted volumes – various methods, differing accuracy levels;
- Manually estimated volumes in lieu of actual counts, as a substitute for counts;
- Manually estimated future volumes based on existing counts plus growth assumptions; and
- Future volumes forecast by computer models based on land use forecasts.

Where future conditions are concerned, forecasts can be generated either by manual projections based on an existing count plus estimated growth trends, or by a traffic forecasting model based on land use. These are two very different methods. The manual method based on counts is common practice with traffic impact studies for individual developments with near term horizon years, since the development being studied usually adds only a small (comparatively) impact to background traffic. The success of this method obviously depends on the quality of the initial count data and the accuracy of the assumed distribution pattern for site impacts. When many developments are combined and a long term horizon year is used, the method loses accuracy because there are multiple interactions between all developments. Background assumptions become very important, and litigation abounds over such issues. Because the method is done by hand, and relies on assumptions to cover the background issues, there is much diversity of results between different analysts.

The traffic model approach treats all developments in a consistent way. Traffic forecasting models also provide the background context by covering the entire city or subarea, not just the development at hand. All input assumptions are land use projections in each individual Traffic Analysis Zones. But traffic models are large, complex systems that need careful calibration in the beginning and expert operation and maintenance thereafter. Such models are also not perfect, but a well-calibrated forecasting model comes close to matching existing counts, when existing land use data is input. That validation test is the only way that counts are used with a forecasting model. After that, it’s all forecast numbers. The best use of forecasting models is to compare one model case to another model case, because that tends to neutralize the calibration differences between the model



and reality. In the DEIS, the evaluation of the Town Center alternatives is based on the comparison to the No Action alternative.

The Sammamish Traffic Forecasting Model was calibrated to closely match 2001 counts, based on the input of 2001 land use data and road network information. This calibration accuracy gives confidence that that model will predict future volumes with similar accuracy. In addition to planning studies, the model is used for concurrency, to track the cumulative effect of adding new development applications to the 2001 land use base. The resulting volume forecasts represent the future condition when all pipeline developments are developed and generating traffic. The 2030 model is based instead on the City's total growth projections to "buildout" based on land use codes (or alternative assumptions) beyond the present day concurrency pipeline.

Do 2006 counts have to do with the traffic model? No. The model calibration to counts was done with 2001 counts and 2001 land use data. The calibration is still valid for this use. The 2006 counts show that growth has happened since 2001, in real terms. The traffic model also shows growth. It forecasts higher volumes for the concurrency future than were true in 2001. But since the concurrency future case includes all development now in the planning/permit/construction pipeline, it goes well beyond existing 2006 conditions. As long as the 2006 counts fall somewhere between the 2001 counts and the concurrency future forecast, the model is working as designed.

Why then are 2006 counts even reported in the DEIS? They are reported to provide the reader with a sense of today's volumes and level of service as a reference. They do not directly serve to help the evaluation of the future alternatives.

## **8. North/South Distribution of Trips**

Commissioner Moran asked the transportation team to clarify the assumption that the majority of traffic, from town center, would head south vs. north, given that it is in the LWSD. The trip distribution pattern for the Town Center site is modeled for the afternoon peak hour, roughly 5 pm or later. Activity at high schools at this time is small compared to the peak hour for each high school that occurs earlier in the day. Travel between the Town Center and Eastlake High School at this hour is nearly negligible. Travel at other hours of the day is accounted for by the peak-to-daily expansion factors on 228th that are used to estimate daily volumes from the peak hour assignment. Existing patterns of orientation to each high school are a constituent part of the existing expansion factors, so the mid-day high school connection to Town Center is actually covered in the forecast daily travel volumes. That said, the high school portion of daily travel patterns is not a dominant part of the total travel activity of any residential area, Town Center or otherwise. For commercial areas, it is even less. For both residential and commercial land uses in the future Town Center, there is a roughly even split of destination opportunities for travel to the south and to the north, with a slightly larger emphasis to the south.

After discounting for the trips internalized within Town Center due to mixed-use effects, the remaining distribution pattern of "exported" trips away from Town Center travels in all directions, with a slightly larger share to the south than to the north. For Town Center Alternative 1, the distribution is 27% north on 228th, 34% south on 228th, 27% west on SE 4th, and 12% east on SE 8th.

The commercial component of Town Center attracts traffic from all directions on the plateau, roughly in proportion to the weight of residential population in each direction. There is also some commercially generated traffic to/from other commercial areas, which are found both north and south along 228th.



The residential component generates traffic that connects to employment opportunities that are mostly outside Sammamish, and to commercial destinations located both within and outside Sammamish. Commuter trips split north and south depending on proximity to the external highway system. At Town Center, slightly more go south to I-90 versus north to SR-202/SR-520. Much of the remainder of residential trip generation is oriented to shopping centers within Sammamish, which are found in both directions from the Town Center area. Finally, there is travel from residences to other residential destinations all over the plateau, and to commercial and residential destinations beyond city limits.



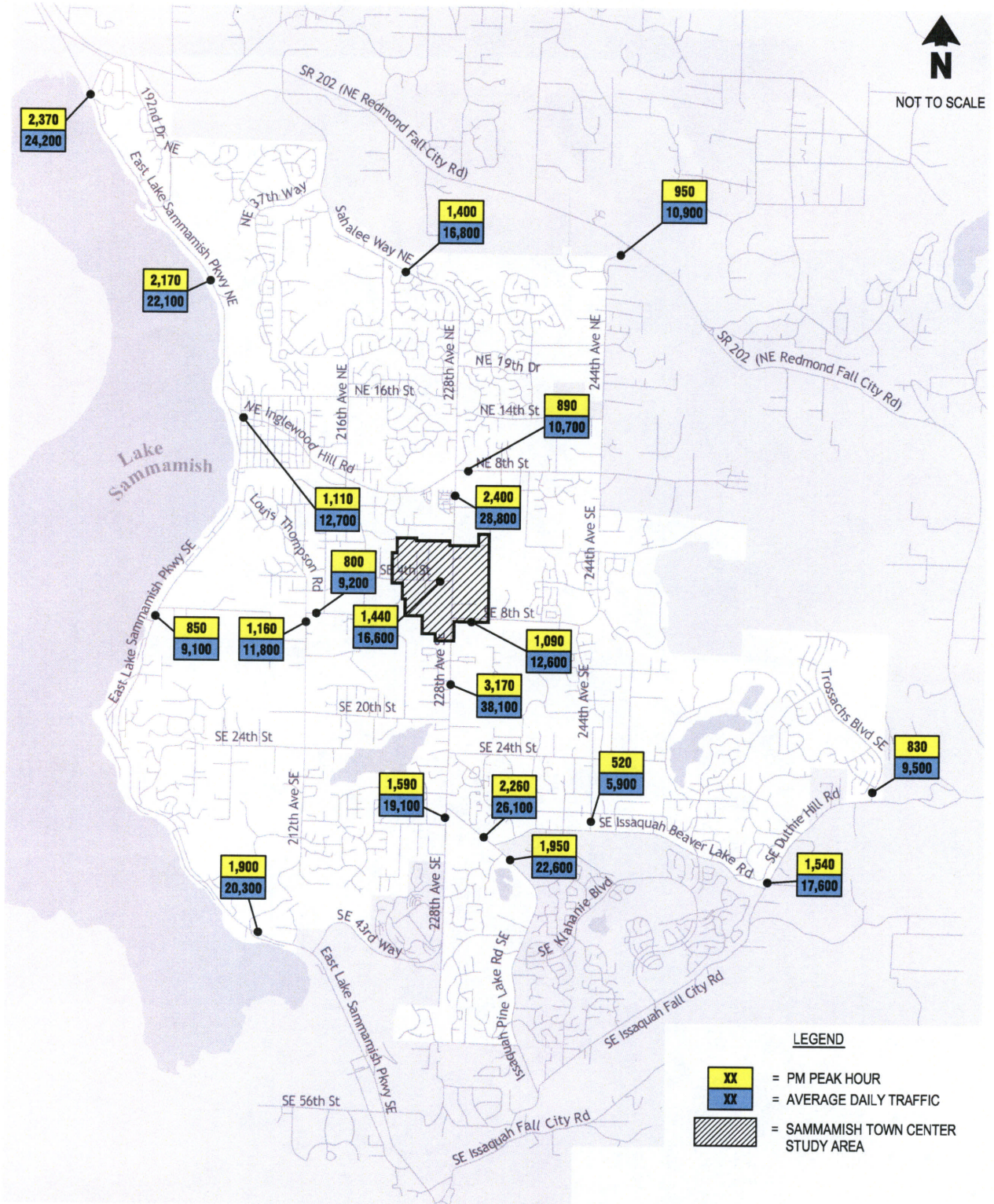
## **Attachment A**

**Updated Traffic Volumes (2006 Existing and Alternatives 1 – 4)**







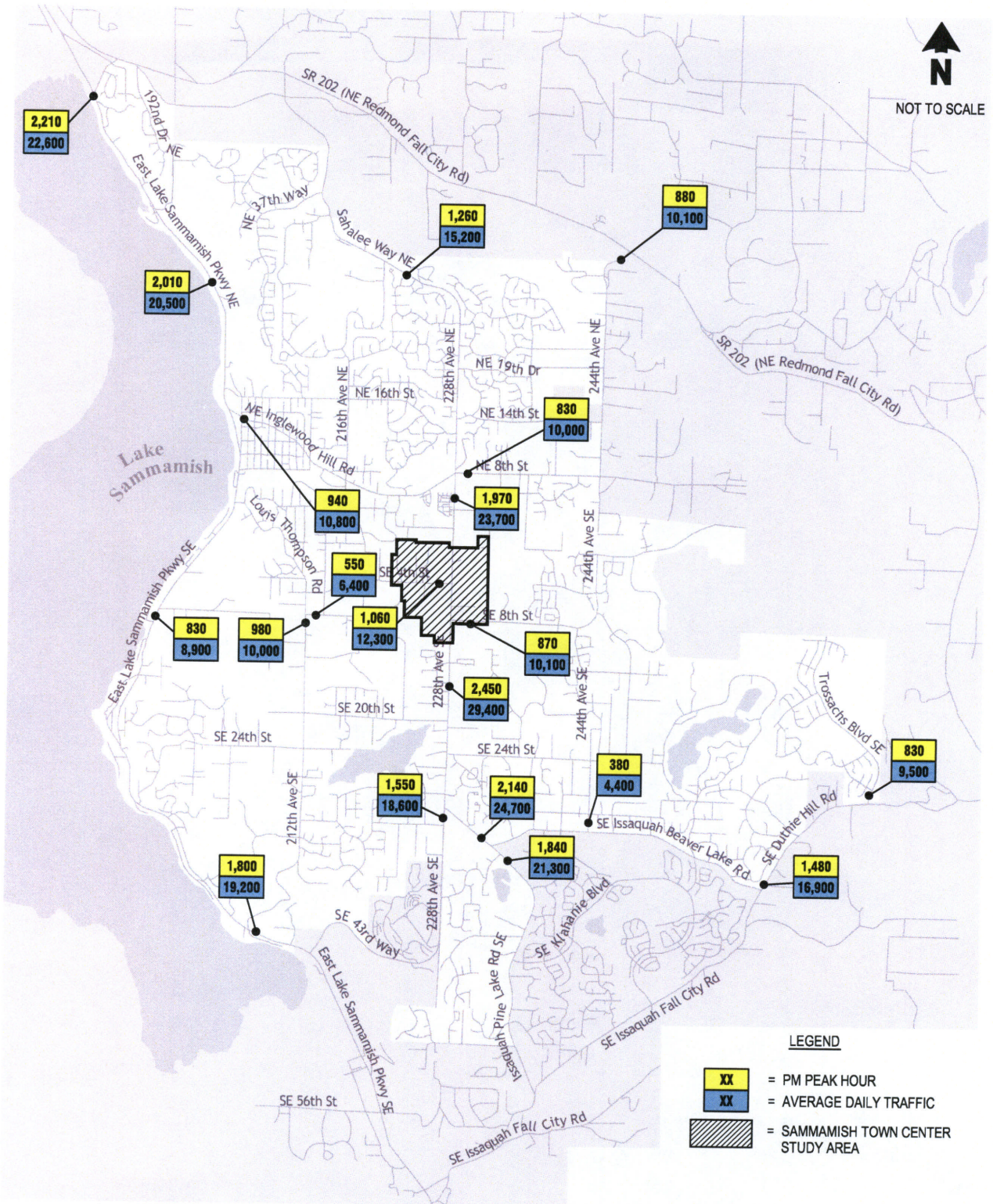


SOURCE: DEA Inc.

2030 Alternative 1 Traffic Volumes (Updated DEIS Figure 7-6)  
 City of Sammamish Town Center Sub-Area Plan DEIS





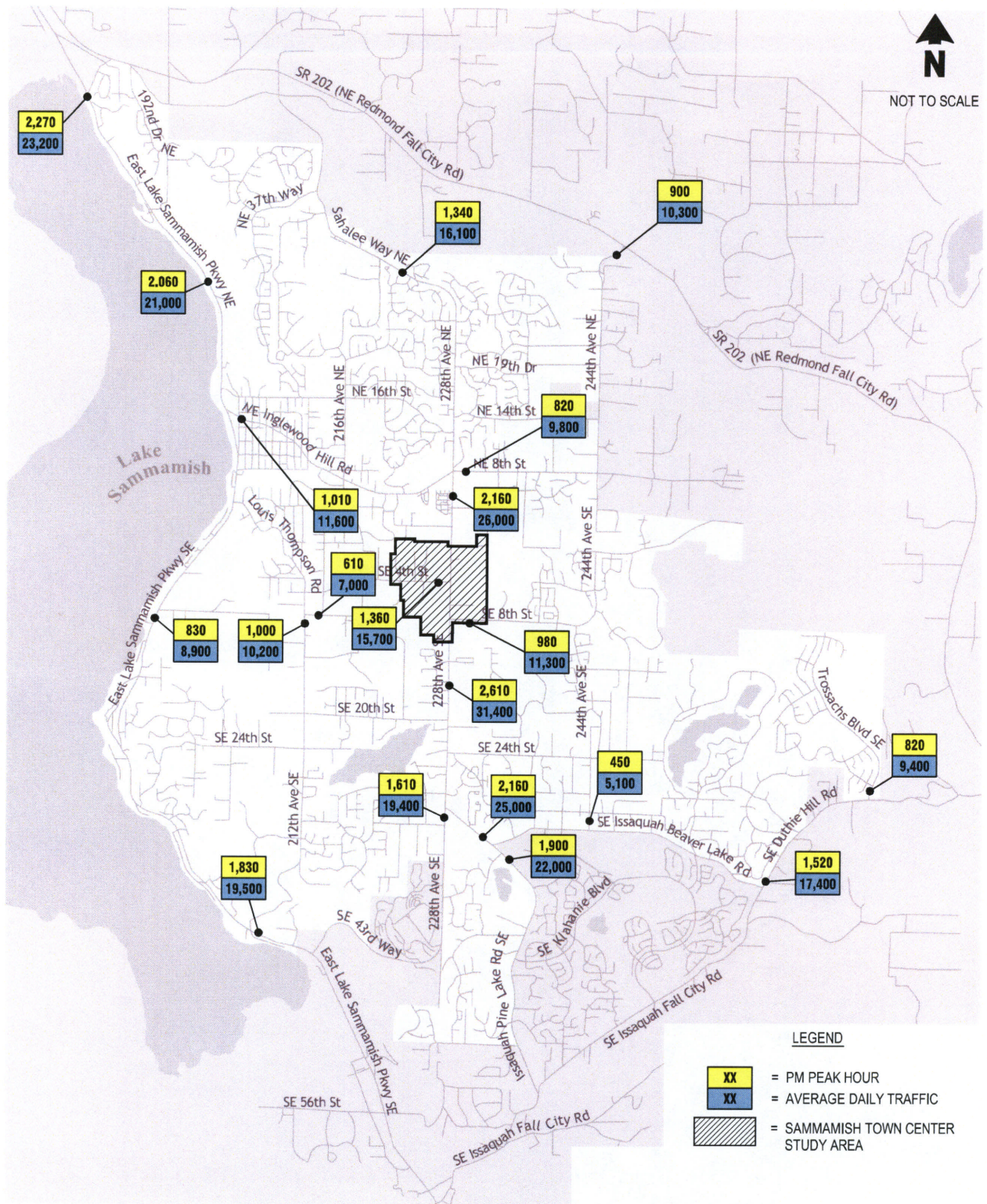


SOURCE: DEA Inc.

2030 Alternative 2 Traffic Volumes (Updated DEIS Figure 7-7)  
 City of Sammamish Town Center Sub-Area Plan DEIS





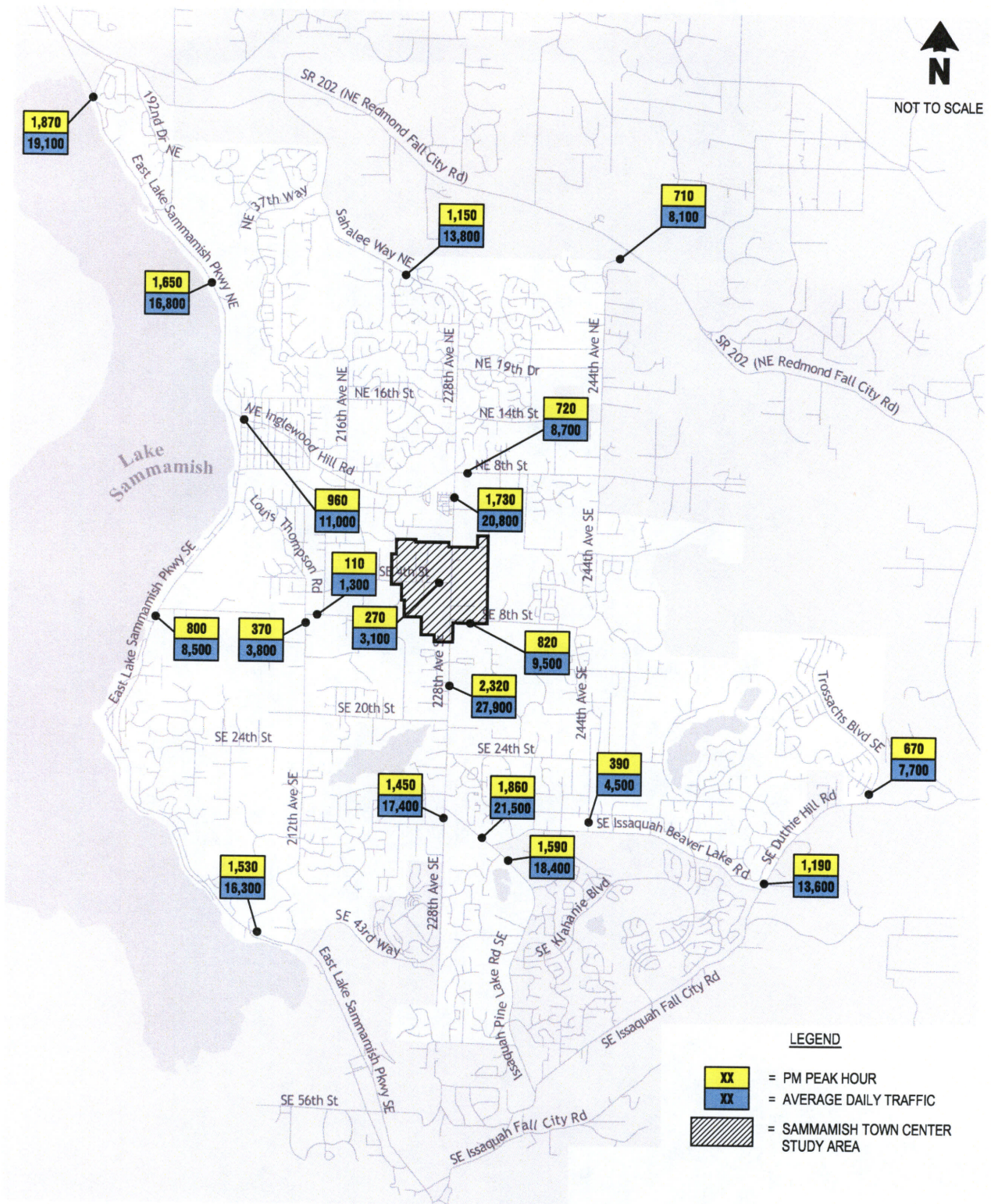


SOURCE: DEA Inc.

2030 Alternative 3 Traffic Volumes (Updated DEIS Figure 7-8)  
 City of Sammamish Town Center Sub-Area Plan DEIS







SOURCE: DEA Inc.

2030 Alternative 4 (No-Action) Traffic Volumes (Updated DEIS Figure 7-9)  
 City of Sammamish Town Center Sub-Area Plan DEIS

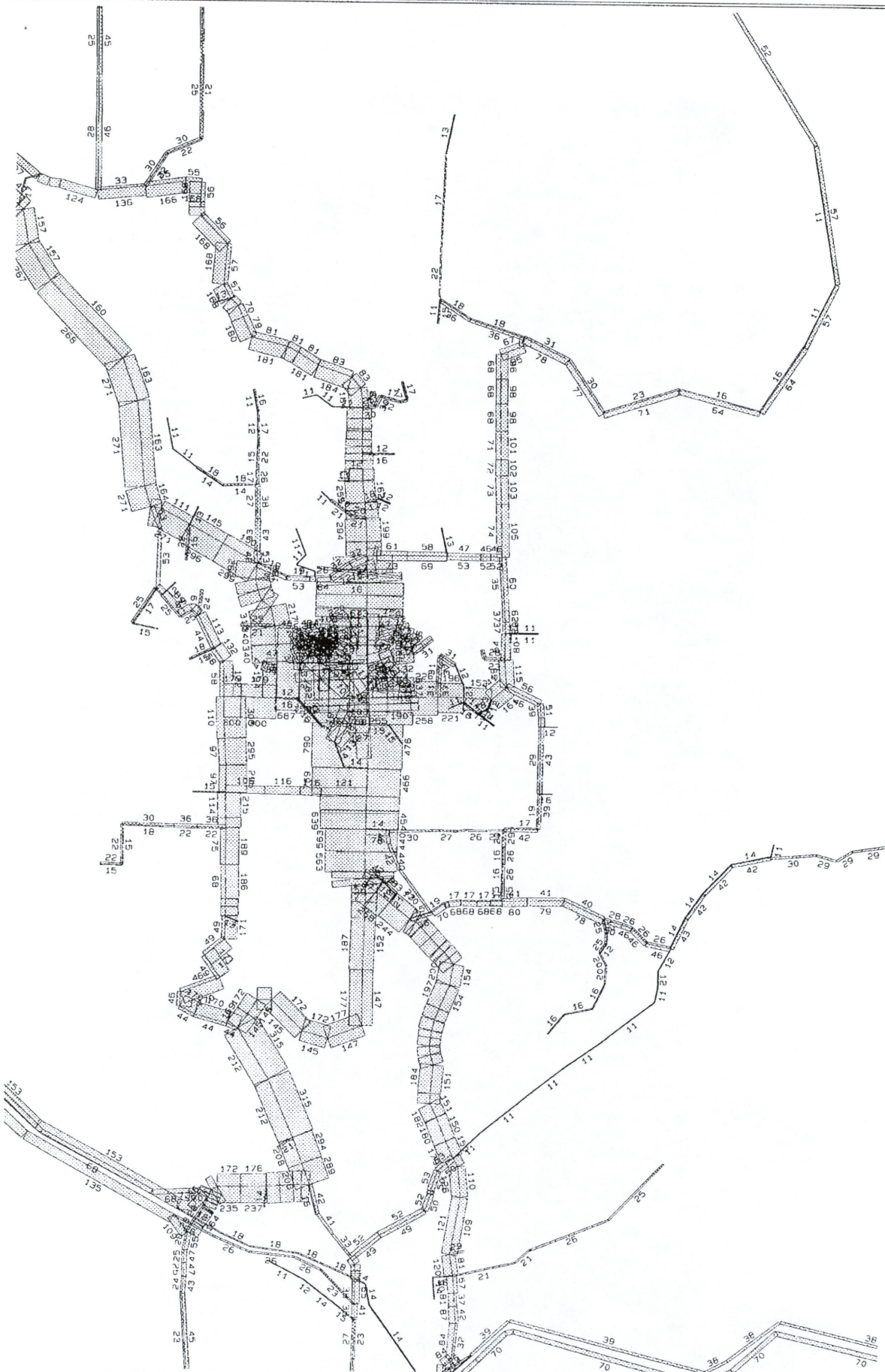




# **Attachment B**

**Total Trip Distribution (Alternatives 1 – 3)**





TMODEL™ 2

# Town Center Alt 1 rev Distribution

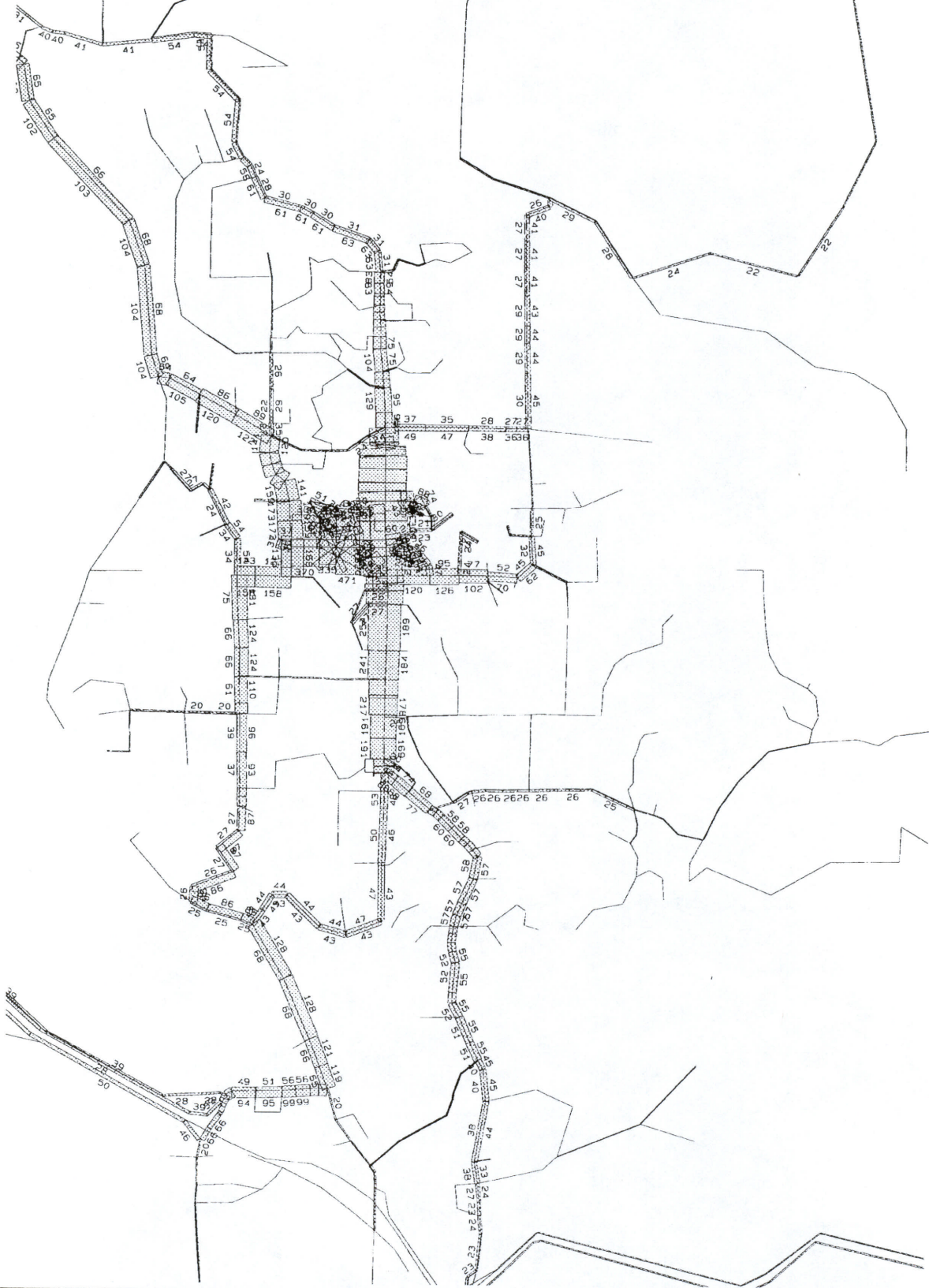
Sammamish Traffic Forecasting Model - 2030 PM Pk Hr

DAVID EVANS & ASSOCIATES, BELLEVUE, WA

TCAL1C.LLX  
TCAL1.NOE

LL: 9500/14000  
UR: 14000/21000  
01-16-2007





TMODEL™ 2

# Town Center Alt 2 rev Distribution

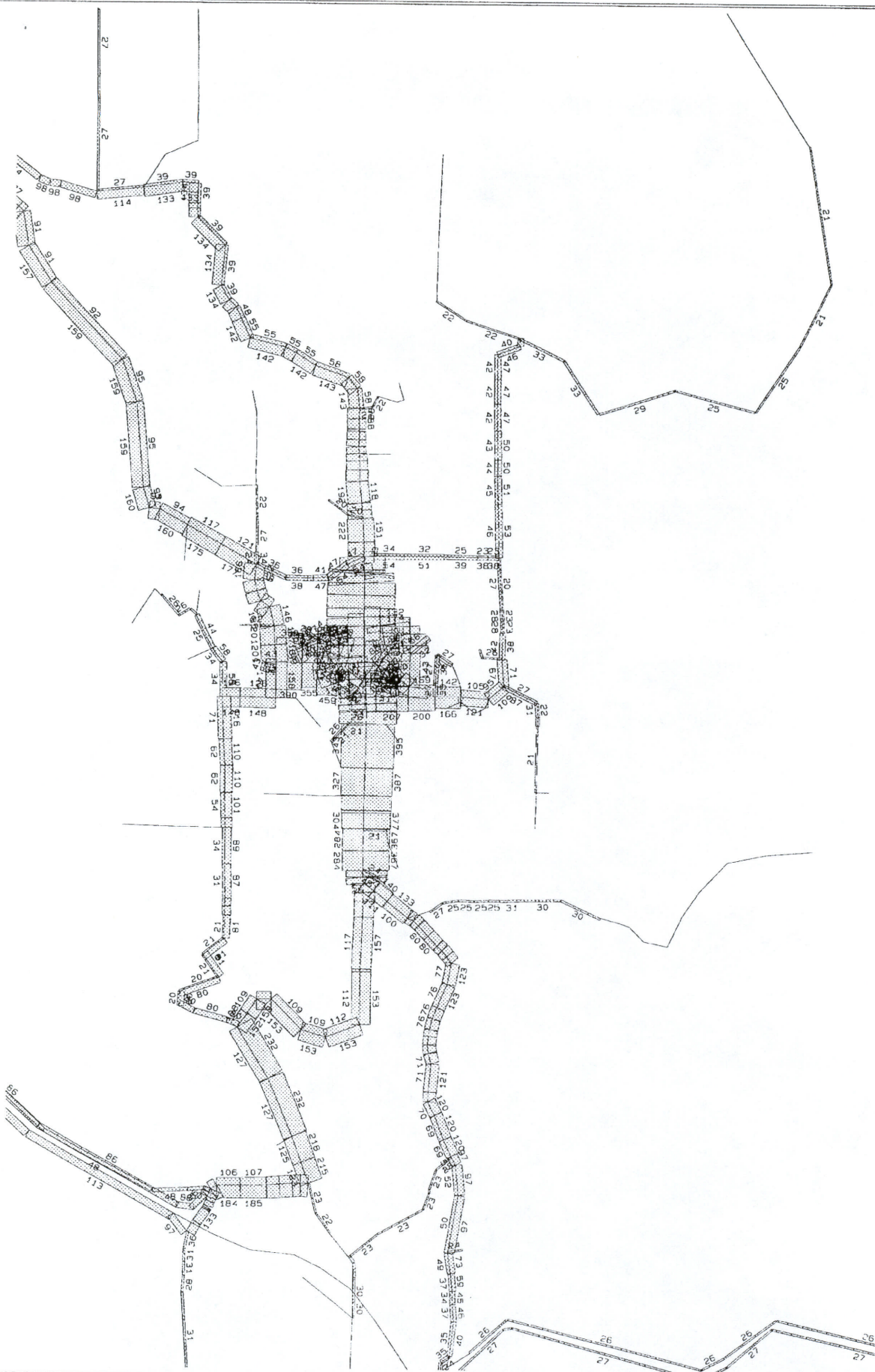
Sammamish Traffic Forecasting Model - 2030 PM Pk Hr

DAVID EVANS & ASSOCIATES, BELLEVUE, WA

TCALT2C.LLX  
TCALT2.NDE

LL: 9500/14000  
UR: 14000/21000  
01-16-2007





*TMODEL™ 2*

# Town Center Alt 3 rev Distribution

Sammamish Traffic Forecasting Model - 2030 PM Pk Hr

DAVID EVANS & ASSOCIATES, BELLEVUE, WA

TCAL13C.LLX  
TCAL13.NDE

LL: 9500/14000  
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01-16-2007

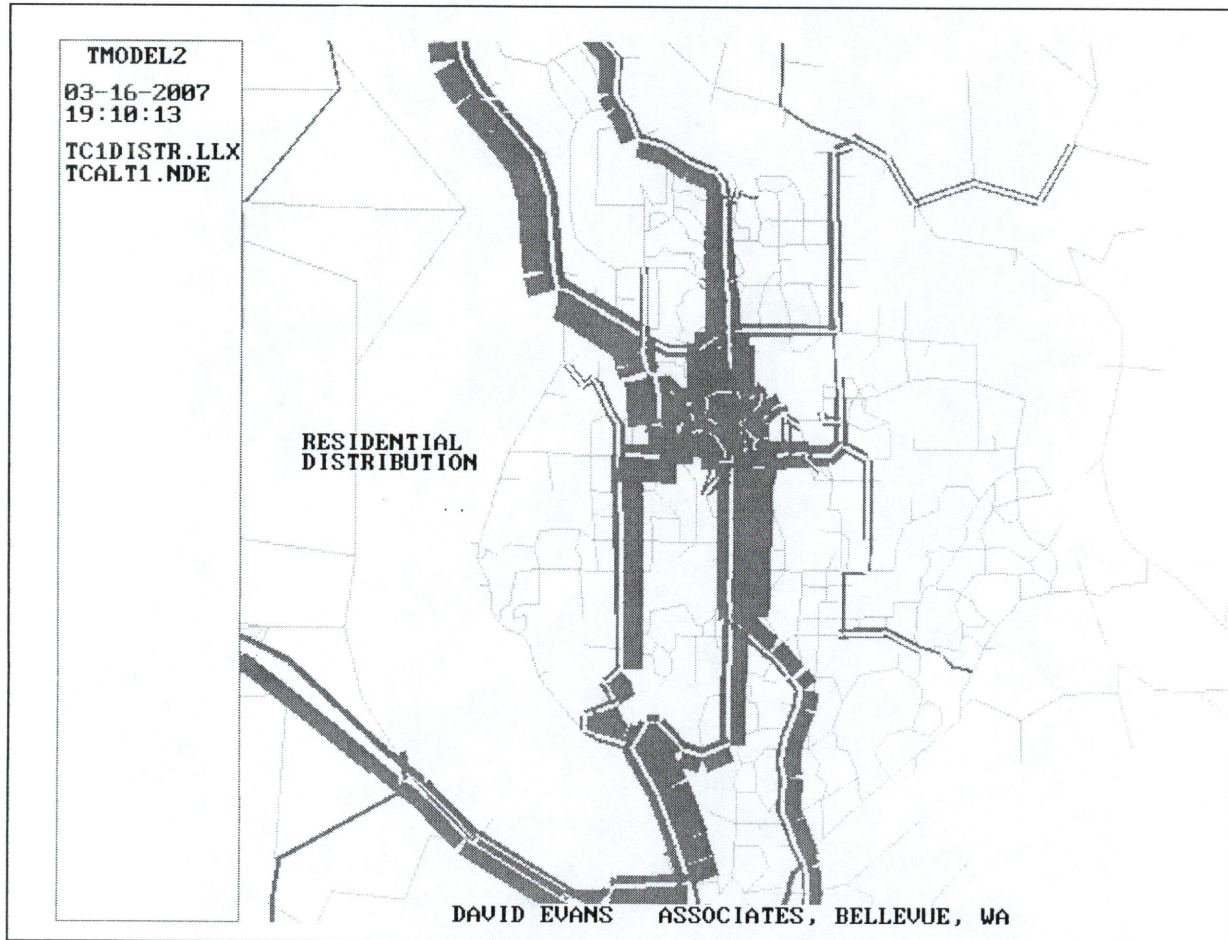


## **Attachment C**

**Trip Distribution by Land Use Type (Residential vs. Non-Residential)**

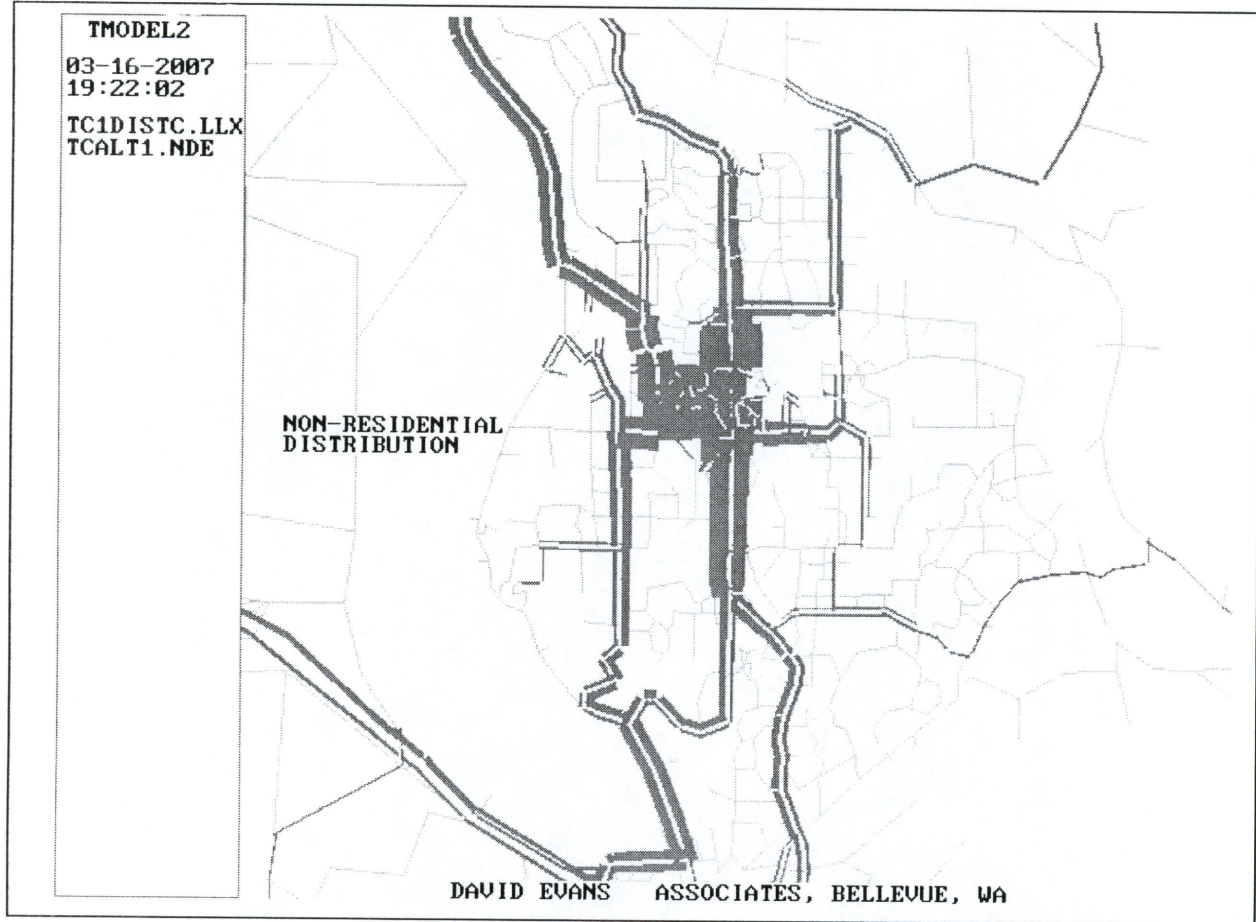


# Trip Distribution: Residential





# Trip Distribution: Non – Residential





# **Attachment D**

## **Existing 2006 Roadway Link Traffic Volumes**



Existing 2006 AM-PM-Average Weekday Daily Trips (AWDT)

| Location               |                       | PM%   | PM    | AM%  | AM    | AWDT   |
|------------------------|-----------------------|-------|-------|------|-------|--------|
| E Lk Sammamish Pkwy NE | s/o 187th             | 9.7%  | 1,742 | 7.9% | 1,425 | 17,949 |
| E Lk Sammamish Pkwy NE | s/o Inglewood Hill Rd | 9.7%  | 1,134 | 7.5% | 871   | 11,650 |
| E Lk Sammamish Pkwy NE | s/o SE 8th St         | 9.5%  | 849   | 7.3% | 656   | 8,950  |
| E Lk Sammamish Pkwy SE | n/o SE 43rd Way       | 8.8%  | 2,770 | 7.6% | 2,415 | 31,610 |
| E Lk Sammamish Pkwy SE | s/o 212th Way SE      | 9.8%  | 1,513 | 7.3% | 1,118 | 15,366 |
| 212th Ave SE           | s/o SE 8th St         | 9.4%  | 444   | 7.0% | 330   | 4,740  |
| 212th Ave SE           | s/o SE 20th St        | 10.2% | 388   | 7.8% | 295   | 3,799  |
| NE Inglewood Hill Rd   | e/o E Lk Samm Pkwy    | 8.7%  | 1,053 | 8.2% | 984   | 12,050 |
| SE 20th St             | w/o 228th Ave SE      | 9.7%  | 461   | 7.2% | 343   | 4,744  |
| Sahalee Way NE         | n/o NE 50th St        | 8.0%  | 1,251 | 6.2% | 982   | 15,735 |
| 228th Avenue NE        | s/o NE 8th St         | 8.4%  | 2,219 | 7.7% | 2,025 | 26,404 |
| 228th Avenue SE        | s/o SE 8th St         | 8.7%  | 2,162 | 7.2% | 1,782 | 24,903 |
| 228th Avenue SE        | n/o SE 32nd St        | 8.6%  | 1,381 | 7.9% | 1,274 | 16,116 |
| 228th Avenue SE        | s/o Issq Pine Lk Rd   | 8.6%  | 1,448 | 7.9% | 1,337 | 16,905 |
| NE 8th St              | e/o 228th Ave NE      | 8.7%  | 1,105 | 8.1% | 1,038 | 12,769 |
| SE 8th St              | e/o 228th Ave SE      | 9.1%  | 893   | 7.3% | 714   | 9,831  |
| Issq Pine Lk Rd SE     | s/o 228th Ave SE      | 9.1%  | 1,689 | 7.7% | 1,436 | 18,646 |
| Issq Pine Lk Rd SE     | n/o 32nd way          | 8.7%  | 1,568 | 7.4% | 1,339 | 18,103 |
| Issq Pine Lk Rd SE     | at Highlands Drive    | 7.9%  | 2,165 | 6.8% | 1,846 | 27,285 |
| 244th Ave NE           | uninc, s/o SR 202     | 8.0%  | 387   | 8.9% | 427   | 4,810  |
| 244th Ave NE           | s/o SE 24th           | 9.3%  | 357   | 7.8% | 301   | 3,853  |
| SE Issq Bvr Lk Rd      | w/o Duthie Hill Rd    | 9.0%  | 209   | 6.7% | 155   | 2,328  |
| SE Duthie Hill Rd      | e/o Bvr Lk Rd         | 8.4%  | 1,116 | 7.3% | 970   | 13,308 |
| Trossachs Blvd SE      | n/o Duthie Hill Rd    | 8.7%  | 665   | 8.0% | 616   | 7,681  |



Model AM-PM-Average Weekday Daily Trips (AWDT) for all Alts

| Segment # | Location               | Alternative 1 |       | Alternative 2 |      | Alternative 3 |        | Alternative 4 |       |        |      |       |        |
|-----------|------------------------|---------------|-------|---------------|------|---------------|--------|---------------|-------|--------|------|-------|--------|
|           |                        | PM            | ADT   | PM            | ADT  | PM            | ADT    | PM            | ADT   |        |      |       |        |
| 1         | E Lk Sammamish Pkwy NE | 9.8%          | 2,370 | 24,200        | 9.8% | 2,210         | 22,600 | 9.8%          | 2,270 | 23,200 | 9.8% | 1,870 | 19,100 |
| 4         | E Lk Sammamish Pkwy NE | 9.4%          | 1,270 | 13,500        | 9.4% | 1,280         | 13,600 | 9.4%          | 1,270 | 13,500 | 9.4% | 1,030 | 11,000 |
| 5         | E Lk Sammamish Pkwy NE | 9.4%          | 850   | 9,100         | 9.4% | 830           | 8,900  | 9.4%          | 830   | 8,900  | 9.4% | 800   | 8,500  |
| 6         | E Lk Sammamish Pkwy SE | 9.4%          | 790   | 8,400         | 9.4% | 780           | 8,300  | 9.4%          | 780   | 8,300  | 9.4% | 780   | 8,300  |
| 8         | E Lk Sammamish Pkwy SE | 9.4%          | 1,900 | 20,300        | 9.4% | 1,800         | 19,200 | 9.4%          | 1,830 | 19,500 | 9.4% | 1,530 | 16,300 |
| 12        | 212th Ave SE           | 9.8%          | 1,160 | 11,800        | 9.8% | 980           | 10,000 | 9.8%          | 1,000 | 10,200 | 9.8% | 370   | 3,800  |
| 13        | 212th Ave SE           | 9.8%          | 870   | 8,900         | 9.8% | 700           | 7,100  | 9.8%          | 740   | 7,500  | 9.8% | 350   | 3,600  |
| 15        | NE Inglewood Hill Rd   | 8.7%          | 1,110 | 12,700        | 8.7% | 940           | 10,800 | 8.7%          | 1,010 | 11,600 | 8.7% | 960   | 11,000 |
| 20        | SE 20th St             | 8.7%          | 610   | 7,100         | 8.7% | 610           | 7,000  | 8.7%          | 590   | 6,800  | 8.7% | 430   | 5,000  |
| 22        | Sahalee Way NE         | 8.3%          | 1,200 | 14,400        | 8.3% | 1,060         | 12,700 | 8.3%          | 1,130 | 13,600 | 8.3% | 910   | 10,900 |
| 24        | 228th Avenue NE        | 8.3%          | 2,400 | 28,800        | 8.3% | 1,970         | 23,700 | 8.3%          | 2,160 | 26,000 | 8.3% | 1,730 | 20,800 |
| 25        | 228th Avenue SE        | 8.3%          | 3,170 | 38,100        | 8.3% | 2,450         | 29,400 | 8.3%          | 2,610 | 31,400 | 8.3% | 2,320 | 27,900 |
| 26        | 228th Avenue SE        | 8.3%          | 3,320 | 39,900        | 8.3% | 3,100         | 37,200 | 8.3%          | 3,190 | 38,400 | 8.3% | 2,670 | 32,100 |
| 27        | 228th Avenue SE        | 8.3%          | 1,590 | 19,100        | 8.3% | 1,550         | 18,600 | 8.3%          | 1,610 | 19,400 | 8.3% | 1,450 | 17,400 |
| 28        | NE 8th St              | 8.3%          | 890   | 10,700        | 8.3% | 830           | 10,000 | 8.3%          | 820   | 9,800  | 8.3% | 720   | 8,700  |
| 29        | SE 8th St              | 8.7%          | 1,090 | 12,600        | 8.7% | 870           | 10,100 | 8.7%          | 980   | 11,300 | 8.7% | 820   | 9,500  |
| 32        | Issq Pine Lk Rd SE     | 8.7%          | 2,260 | 26,100        | 8.7% | 2,140         | 24,700 | 8.7%          | 2,160 | 25,000 | 8.7% | 1,860 | 21,500 |
| 33        | Issq Pine Lk Rd SE     | 8.7%          | 1,950 | 22,600        | 8.7% | 1,840         | 21,300 | 8.7%          | 1,900 | 22,000 | 8.7% | 1,590 | 18,400 |
| 34        | Issq Pine Lk Rd SE     | 8.7%          | 2,590 | 29,900        | 8.7% | 2,510         | 29,000 | 8.7%          | 2,530 | 29,300 | 8.7% | 2,230 | 25,800 |
| 35        | 244th Ave NE           | 8.7%          | 950   | 10,900        | 8.7% | 880           | 10,100 | 8.7%          | 900   | 10,300 | 8.7% | 710   | 8,100  |
| 39        | 244th Ave NE           | 8.7%          | 520   | 5,900         | 8.7% | 380           | 4,400  | 8.7%          | 450   | 5,100  | 8.7% | 390   | 4,500  |
| 42        | SE Issq Bvr Lk Rd      | 8.7%          | 560   | 6,400         | 8.7% | 410           | 4,700  | 8.7%          | 470   | 5,400  | 8.7% | 330   | 3,800  |
| 43        | SE Duthie Hill Rd      | 8.7%          | 1,540 | 17,600        | 8.7% | 1,480         | 16,900 | 8.7%          | 1,520 | 17,400 | 8.7% | 1,190 | 13,600 |
| 45        | Trossachs Blvd SE      | 8.7%          | 830   | 9,500         | 8.7% | 830           | 9,500  | 8.7%          | 820   | 9,400  | 8.7% | 670   | 7,700  |



**Attachment E**

**Level of Service (LOS) and Traffic Volumes as Reported in the Eastside Catholic  
High School EIS**



**Table 4. Existing levels of service.**

| Signalized Intersections   | AM Peak Hour |                          |           | Noon Peak Hour |                          |           | PM Peak Hour of School |                          |           | PM Peak Hour of Adjacent Streets |                          |           | Special Event Peak Hour |                          |           |
|--|--------------|--------------------------|-----------|----------------|--------------------------|-----------|------------------------|--------------------------|-----------|----------------------------------|--------------------------|-----------|-------------------------|--------------------------|-----------|
|  | LOS          | Delay <sup>a</sup>       | V/C       | LOS            | Delay <sup>a</sup>       | V/C       | LOS                    | Delay <sup>a</sup>       | V/C       | LOS                              | Delay <sup>a</sup>       | V/C       | LOS                     | Delay <sup>a</sup>       | V/C       |
| SR 202 / East Lake Sammamish Parkway                                 | F            | 80.8                     | 1.00      | -              | -                        | -         | D                      | 37.5                     | 0.66      | D                                | 42.0                     | 0.84      | -                       | -                        | -         |
| SR 202 / 192 <sup>nd</sup> Drive NE                                  | B            | 13.5                     | 0.82      | -              | -                        | -         | A                      | 6.4                      | 0.64      | A                                | 7.7                      | 0.75      | -                       | -                        | -         |
| SR 202 / Sahalee Way NE  | C            | 27.7                     | 0.86      | -              | -                        | -         | C                      | 29.6                     | 0.88      | F                                | 104.4                    | 1.13      | -                       | -                        | -         |
| NE 37 <sup>th</sup> Way / Sahalee Way NE                             | A            | 9.1                      | 0.56      | -              | -                        | -         | A                      | 7.6                      | 0.43      | A                                | 9.0                      | 0.53      | -                       | -                        | -         |
| NE 25 <sup>th</sup> Way / Sahalee Way NE                             | B            | 14.4                     | 0.55      | B              | 10.0                     | 0.41      | B                      | 11.9                     | 0.50      | B                                | 12.2                     | 0.51      | B                       | 11.0                     | 0.52      |
| NE Eighth Street (Inglewood Hill Road) / 228 <sup>th</sup> Avenue NE | D            | 41.9                     | 0.84      | C              | 24.2                     | 0.62      | C                      | 31.8                     | 0.74      | C                                | 33.1                     | 0.74      | C                       | 34.4                     | 0.81      |
| NE Fourth Street / 228 <sup>th</sup> Avenue                          | C            | 32.8                     | 0.80      | B              | 16.5                     | 0.65      | E                      | 71.1                     | 0.98      | B                                | 18.2                     | 0.75      | B                       | 18.4                     | 0.72      |
| SE Eighth Street / 228 <sup>th</sup> Avenue SE                       | C            | 20.3                     | 0.42      | B              | 11.8                     | 0.33      | B                      | 12.8                     | 0.35      | A                                | 8.3                      | 0.41      | A                       | 9.2                      | 0.44      |
| SE 20 <sup>th</sup> Street / 228 <sup>th</sup> Avenue SE             | B            | 14.6                     | 0.55      | A              | 6.5                      | 0.29      | B                      | 10.3                     | 0.48      | B                                | 10.6                     | 0.46      | B                       | 10.1                     | 0.56      |
| SE 24 <sup>th</sup> Street / 228 <sup>th</sup> Avenue SE             | C            | 22.9                     | 0.64      | B              | 15.2                     | 0.32      | C                      | 24.6                     | 0.52      | C                                | 21.1                     | 0.55      | C                       | 23.2                     | 0.68      |
| Issaquah-Pine Lake Road / 228 <sup>th</sup> Avenue SE                | C            | 32.5                     | 0.83      | C              | 22.4                     | 0.39      | C                      | 29.4                     | 0.52      | C                                | 22.6                     | 0.50      | B                       | 14.4                     | 0.47      |
| Issaquah-Pine Lake Road / Issaquah-Fall City Road                    | C            | 29.5                     | 0.72      | -              | -                        | -         | B                      | 17.4                     | 0.65      | B                                | 17.7                     | 0.72      | -                       | -                        | -         |
| SE 56 <sup>th</sup> Street / East Lake Sammamish Parkway             | E            | 66.0                     | 1.08      | -              | -                        | -         | C                      | 34.8                     | 0.81      | D                                | 44.7                     | 0.87      | -                       | -                        | -         |
| SE 43 <sup>rd</sup> Way / East Lake Sammamish Parkway                | B            | 14.9                     | 0.71      | -              | -                        | -         | B                      | 10.5                     | 0.62      | B                                | 13.2                     | 0.75      | -                       | -                        | -         |
| 212 <sup>th</sup> Way SE / East Lake Sammamish Parkway               | B            | 12.0                     | 0.68      | -              | -                        | -         | A                      | 5.1                      | 0.40      | A                                | 5.1                      | 0.50      | -                       | -                        | -         |
| Inglewood Hill Road / East Lake Sammamish Parkway                    | D            | 36.7                     | 0.88      | -              | -                        | -         | A                      | 9.0                      | 0.54      | B                                | 18.9                     | 0.74      | -                       | -                        | -         |
| <b>Unsignalized Intersections<sup>b</sup></b>                        | <b>LOS</b>   | <b>Delay<sup>a</sup></b> | <b>WM</b> | <b>LOS</b>     | <b>Delay<sup>a</sup></b> | <b>WM</b> | <b>LOS</b>             | <b>Delay<sup>a</sup></b> | <b>WM</b> | <b>LOS</b>                       | <b>Delay<sup>a</sup></b> | <b>WM</b> | <b>LOS</b>              | <b>Delay<sup>a</sup></b> | <b>WM</b> |
| Main Street / 228 <sup>th</sup> Avenue SE                            | C            | 18.5                     | WB        | C              | 22.2                     | WB        | C                      | 24.6                     | WB        | C                                | 16.1                     | WB        | E                       | 39.6                     | WB        |
| SE Fourth Street / 228 <sup>th</sup> Avenue SE                       | C            | 21.4                     | EB        | D              | 25.4                     | EB        | D                      | 25.0                     | EB        | F                                | 61.6                     | EB        | E                       | 38.5                     | EB        |
| Louis Thompson Road / East Lake Sammamish Parkway                    | C            | 16.6                     | WBLT      | -              | -                        | -         | C                      | 22.2                     | WBLT      | E                                | 47.5                     | WBLT      | -                       | -                        | -         |
| SE 20 <sup>th</sup> Street / 212 <sup>th</sup> Avenue SE             | B            | 11.2                     | WB        | -              | -                        | -         | -                      | -                        | -         | B                                | 12.7                     | WB        | -                       | -                        | -         |

EB – eastbound.  
 LOS – level of service.  
 V/C – volume-to-capacity ratio.  
 WB – westbound.  
 WBLT – westbound left turn.  
 WM – worst movement.  
<sup>a</sup> Average delay in seconds per vehicle.  
<sup>b</sup> LOS and delay are reported for the worst movement at unsignalized intersections.







**Attachment F**  
**Joseph P. Savage Jr., P.E. Letter**



MEMORANDUM

Date: March 8, 2007

To: Mr. Scott Hamilton

From: Joseph P. Savage, Jr., P.E.

Per your request, I have reviewed the information regarding the Draft EIS on the Sammamish Town Center, including your initial comment letter and the response from Dan McKinney of The Transpo Group. Although I have been away from Sammamish for over two years, I am still quite familiar with the City and the Comprehensive Plan.

To begin with Mr. McKinney is correct about the ITE Trip Generation estimate of about 10% of daily traffic to/from a single family residential development occurring in the peak hour is not a good estimate of the peak hour percentage of daily traffic on a given street in Sammamish. This is true because those streets carry a mix of residential and non-residential trips, and even the residential peak hour percentages would likely vary from one subdivision to another. Since all you had available for the "existing" traffic volumes were daily traffic, assuming a constant 10% factor is not inappropriate for a non-traffic engineer to begin reviewing the EIS document.

Since Mr. McKinney clearly states that the actual traffic modeling and analysis was conducted using PM peak hour volumes on street segments (links) and intersection approaches, then the EIS should provide the 2006 "existing" PM peak hour traffic volumes used in the EIS as the starting point for analysis. These volumes should be listed in tabular format at the 25 - 30 locations you identified for 2006, and for 2030 for each of the four alternatives (1-3 plus no action). Then one can compare "apples-to-apples." Although I agree with presenting estimated daily traffic volumes based on factoring from the modeled peak hour volumes, all analysis of levels of service at intersections and on street segments should be performed with peak hour rather than daily volumes. Clearly the PM peak hour volumes are readily available for 2006 and the alternatives since Transpo used that data.

I strongly disagree with Mr. McKinney's assertion that "The PM Peak hour intersection operations [I believe he means the LOS calculations] were based on intersection volumes and not the PM peak hour link volumes shown in the figures. So, the operational analysis will not change or be impacted by updating these figures." [Emphasis added.] First, the sums of the traffic volumes entering and leaving a given intersection should reasonably match the directional peak hour link volumes feeding the four legs of the intersection. Thus, there should be a good correlation between the link volumes and the intersection volumes; they are not independent as Mr. McKinney implies. Second, if a comparison of the link volumes between 2006 and 2030, and across the 2030 alternatives, yields questionable results, then both intersection and link volumes should be rationalized.

Looking past the 10% peak hour assumption, your table of 2002, 2006 and 2030 daily traffic volumes appropriately raises significant questions about the accuracy and reliability of the Sammamish traffic model and Transpo's application of it for this DEIS. Mr. McKinney offers no explanation of why the 2002/3 and 2006 daily traffic volumes are different, nor does he explain why 2030 daily traffic volumes on some streets are less than the 2002/3 counts. These



are appropriate questions to be addressed in an EIS, and on their face raise valid questions about the reasonableness of the model results and the conclusions drawn from them.

Having lived in Eagle Ridge adjacent to East Lake Sammamish Parkway (ELSP) for nearly 20 years, and having conducted extensive forensic analysis of the King County traffic models, it is inconceivable to me that ELSP would have such little growth in traffic between 2006 and 2030 – about 1% per year for Alternative 1 and less for the other alternatives. For the No Action Alternative, the analysis shows an actual decline in traffic on ELSP at Weber Point and at SE 8<sup>th</sup> Street. Similarly, 2030 volumes are projected to be less than 2006 volumes on Inglewood Road, NE 8<sup>th</sup>, NE 228<sup>th</sup>, etc. for one or more alternatives or no action. Unless there is a recession in the Puget Sound region, it is not reasonable to expect such declines or low growth situations.

Again, these trends raise serious questions about the reasonableness of the Sammamish traffic model or about Transpo's application of it.

“Calibration” of the traffic model is referenced several times by Mr. McKinney. Is a model calibration report available? Does the model meet FHWA model calibration/validation standards? This information would have been prepared by the model developers (DEA?); I would not expect Transpo to revalidate/recalibrate the model but rather to simply apply what they had been given to work with. Therefore, the questions I raise probably pertain more to the original model rather than Transpo's application of it to the Town Center proposal.

A quick check of the overall validity of the Sammamish model would be to draw a “cordon line” around the city and tally the actual base year traffic counts, then do the same for the 2030 no action alternative and see if the comparison makes sense in relation to projected population and employment growth in Sammamish between the base year and 2030. If the results of that check are not reasonable (i.e., growth of total traffic crossing the cordon line approximates growth in population and employment), then clearly the results of the analysis conducted for the Town Center proposal using that model cannot be relied upon as the basis for a decision on Town Center alternatives by the City.

In closing, the points that you raised in your table and in your list of questions are valid points and deserve an adequate response. The unreasonableness of the traffic model projections for 2030 in comparison to 2006 cause me to question the validity of the entire analysis and results presented in the Draft EIS, even though the problems may lie with the City's traffic model and not the project-specific analysis for the Town Center alternatives. These questions can be addressed by providing the 2006 PM peak hour volumes alongside the forecast 2030 PM peak hour volumes, and the reasonableness of their relationships determined.

Other points, such as the varying PM peak hour percentages by location and by alternative, also deserve attention, but not until the overall reasonableness of the traffic modeling is assured.